

Visteon Corporate Headquarters Village Center



Jamison David Morse
AE 882 – Senior Thesis
Structural Option

➔ I. Introduction

II. Problem Statement

III. Solution Statement

VI. Structure

A. Gravity Framing

B. Lateral System

V. Architecture

VI. Overall Benefits

VII. Conclusions



Image courtesy of SmithGroup

Location: Van Buren, MI
Owner: Visteon Corporation
Arch/Eng/Site: SmithGroup
Type: Design-Build

Cost: \$85 Million
Size: 130,000 gsf
Completed December 2004



Image courtesy of Google

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Image courtesy of SmithGroup

- Composite steel framing
- Special steel moment frame lateral system
- Deep foundation system with HP shape friction piles and concrete pile caps



Image courtesy of SmithGroup

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Image courtesy of SmithGroup

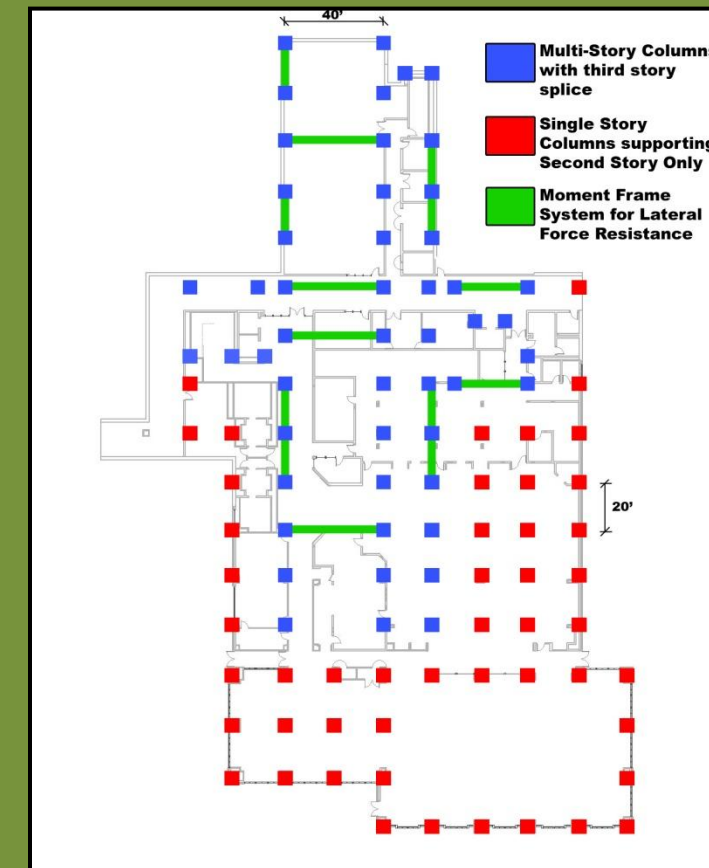


Image courtesy of Jamison Morse

- Single story columns
- Multi story columns
- Six moment frames in E-W
- Five moment frames in N-S

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Image courtesy of SmithGroup

-Relocation of the Visteon Village Center to Orinda, CA

-Increased seismic loading

-Structural redesign

-Possible negative effects on architecture



Image courtesy of Geology.com

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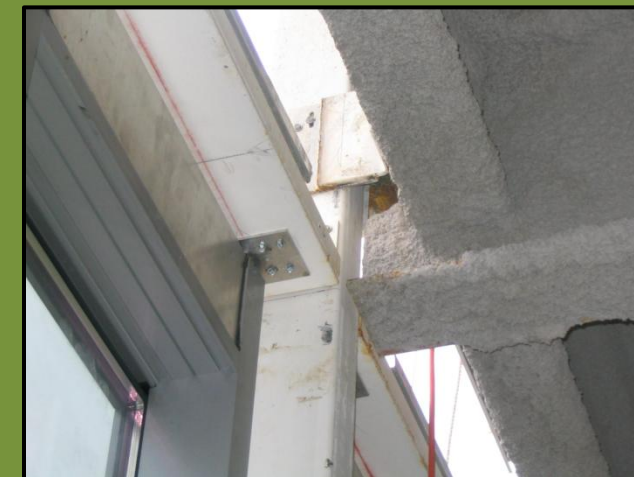


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- Determine critical load case
- Adequacy of current system
- Analysis of different framing schemes
- Integration into architecture
- Benefits of systems

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- Forty foot spans difficult to maintain under new critical loading
- Column grid optimization
- Different framing systems analyzed
- Braced frame systems determined most efficient to resist the critical seismic load case

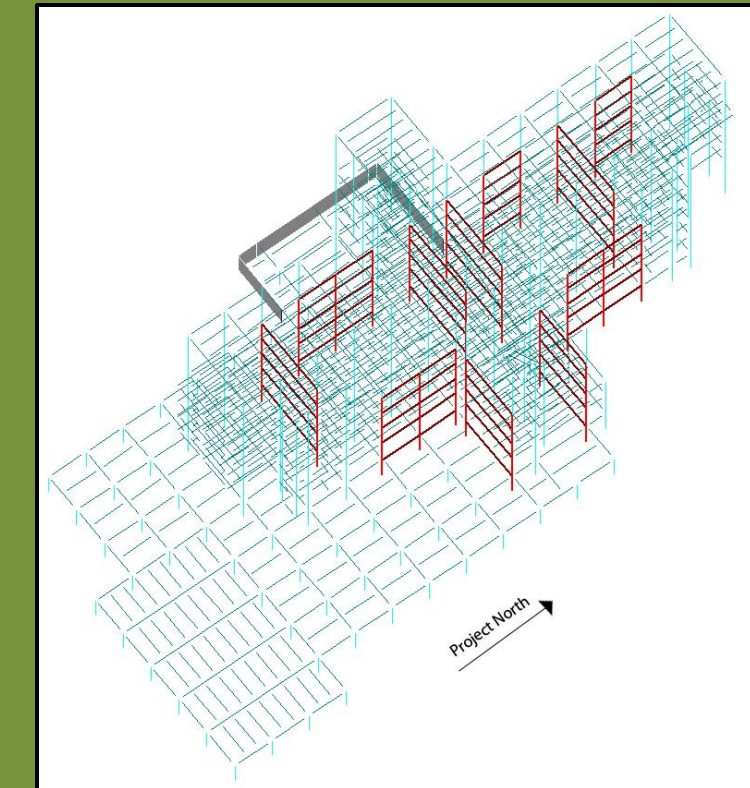


Image courtesy of Jamison Morse

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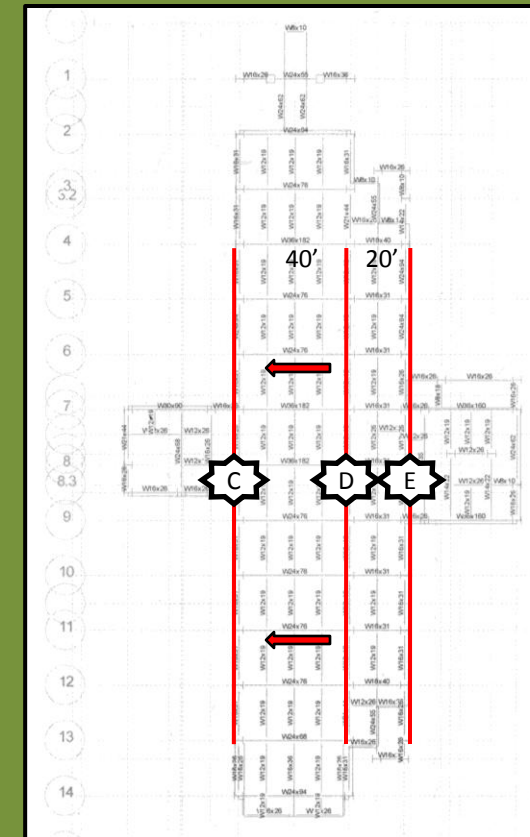


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-Column grid line spacing was originally 40' between C and D and 20' between D and E

-It was decided to move grid line D 10' west to create two equally spaced column grids of 30' each between column lines 4 and 13

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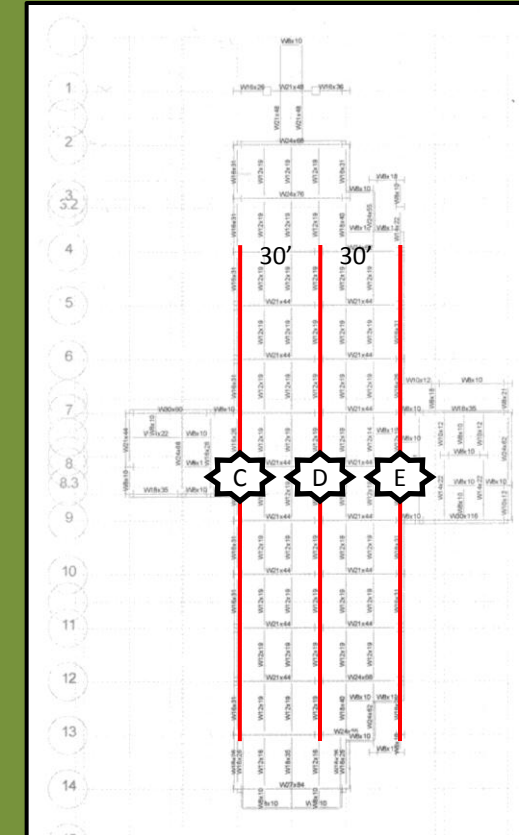


Image courtesy of Jamison Morse

- Much more efficient spacing to utilize for lateral framing system
- Symmetric bays and framing members improves efficiency

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Image courtesy of SmithGroup

- Architectural implications
- Special moment frame drift issues
- Braced frames
 - Eccentrically braced frames
 - Centrically braced frames
- Architectural impact



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- Frames could be integrated architecturally
- Placed to minimize conflicts
- Three 30' wide frames in E-W
- Four 20' wide frames in N-S

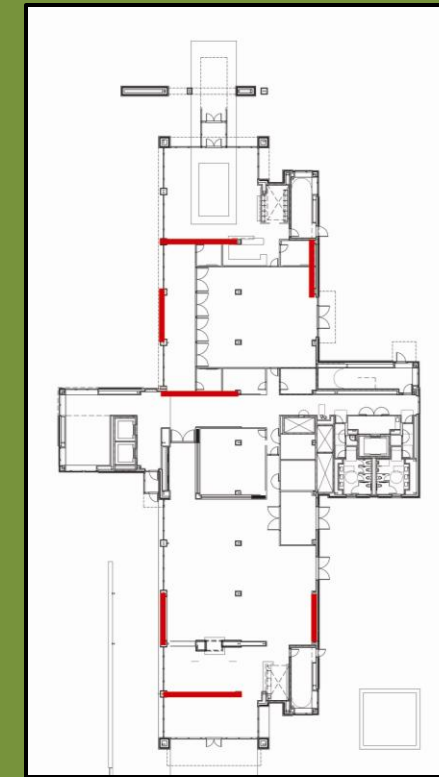


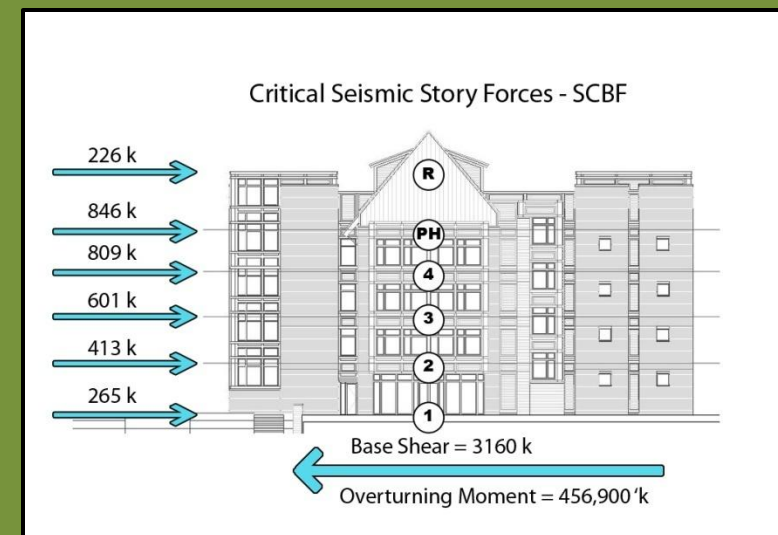
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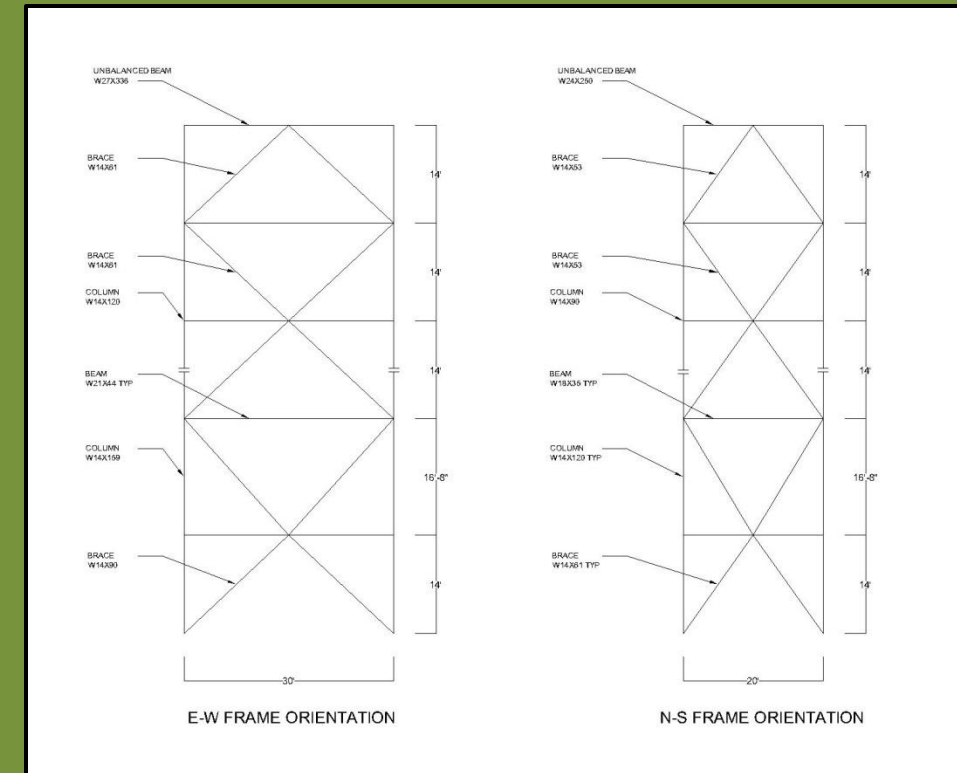


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Using ASCE 7-05:



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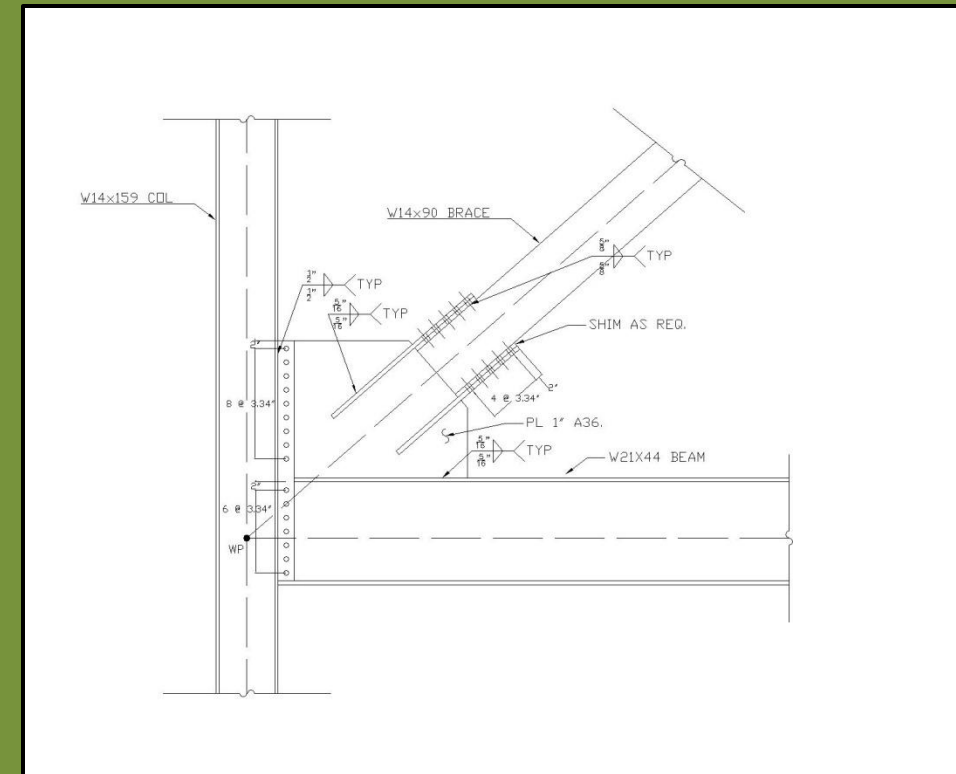


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- AISC Seismic Design Manual
- Detailed R=6 connection
- Wide flange brace

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Buckling Restrained Braced Frames

- Braces designed to have compression strength equal to yield stress
- Connection of $R=8$
- More expensive braces
- Smaller column sizes
- Cheaper connections
- No greater architectural impact

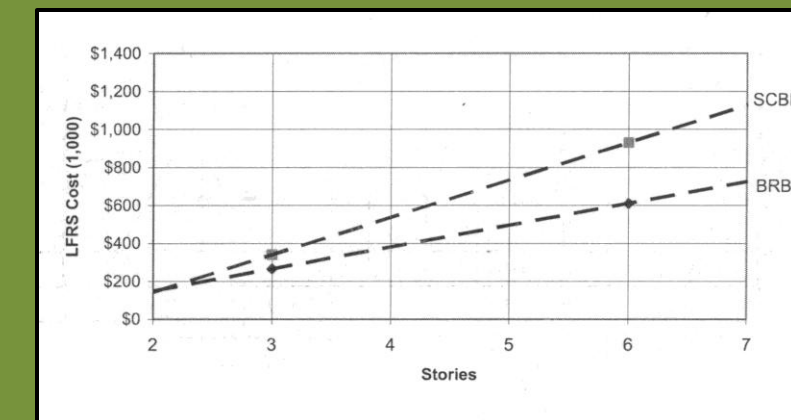


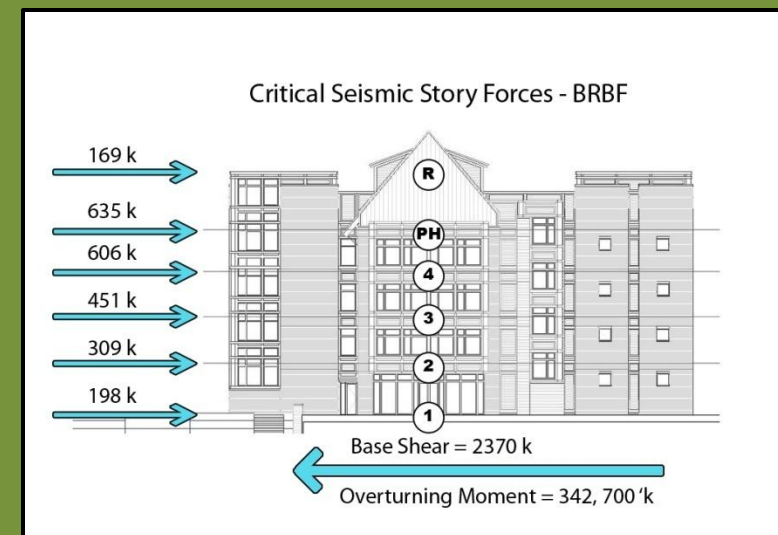
Image courtesy of Thornton Tomasetti

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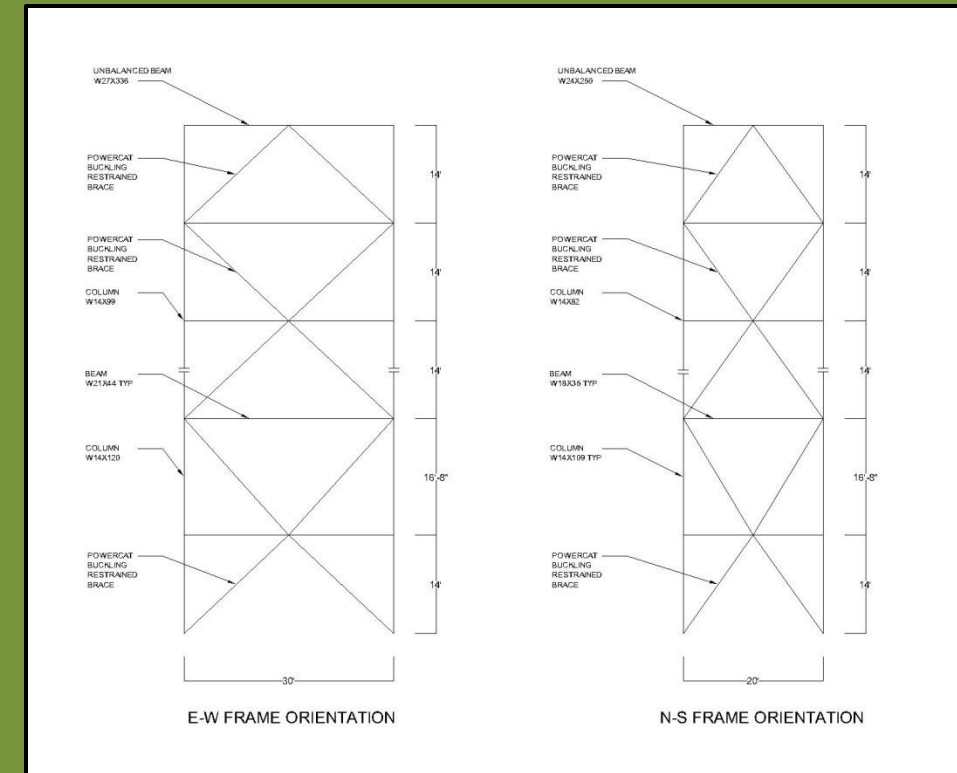


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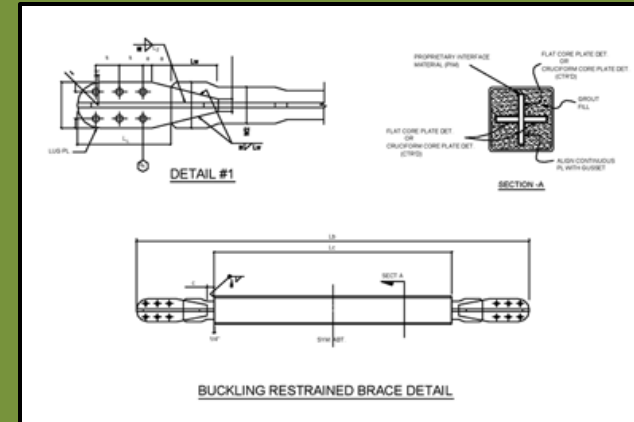


Image courtesy of CoreBrace



Image courtesy of Star Seismic



Image courtesy of Nippon Steel

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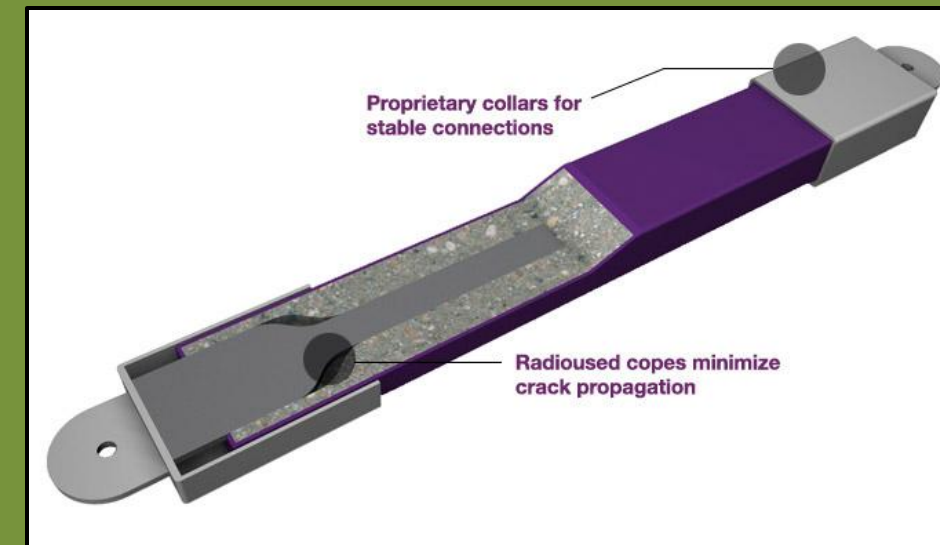


Image courtesy of Star Seismic

- Star Seismic brace chosen
- PowerCat Model
 - Efficient connections
 - Minimal material
 - No gusset stiffeners
 - Easy erection
 - 15% reduction in overall steel cost

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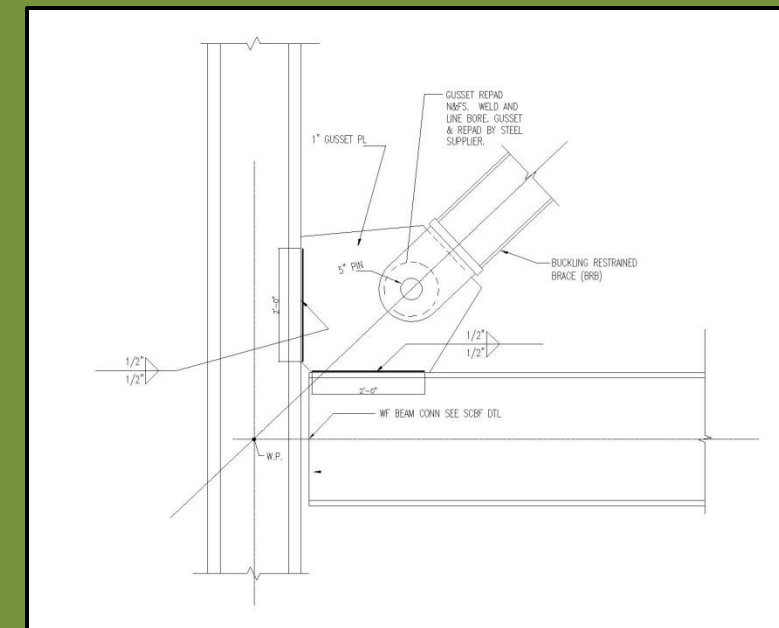


Image courtesy of Star Seismic

- Connections sized for expected yield
- RMS analysis done for impact on foundations

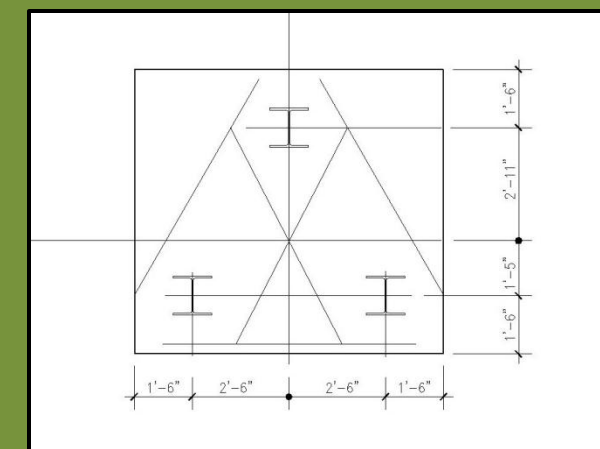


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Image courtesy of SmithGroup

- Concurrent architectural breadth study performed during lateral system design
- Layout conducive to openings
- Small rough openings of 6' wide x 8' tall
- Large rough openings of 12' wide x 8' tall
- Feasibly integrated into building with chosen frame locations

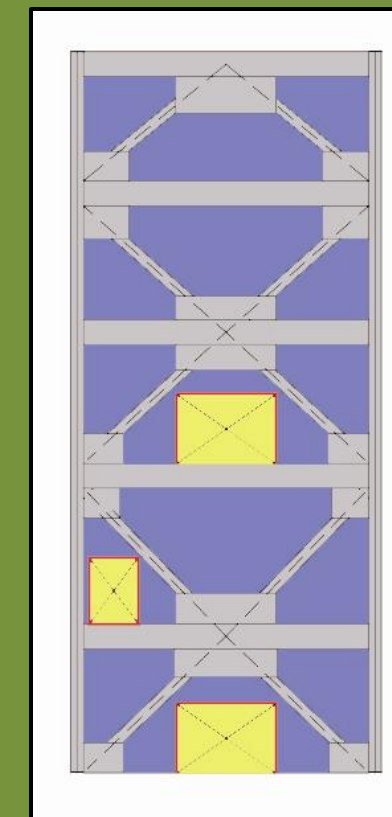
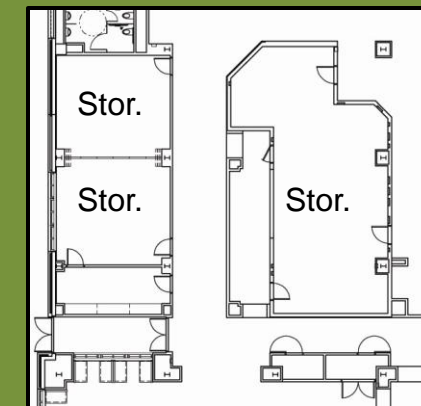
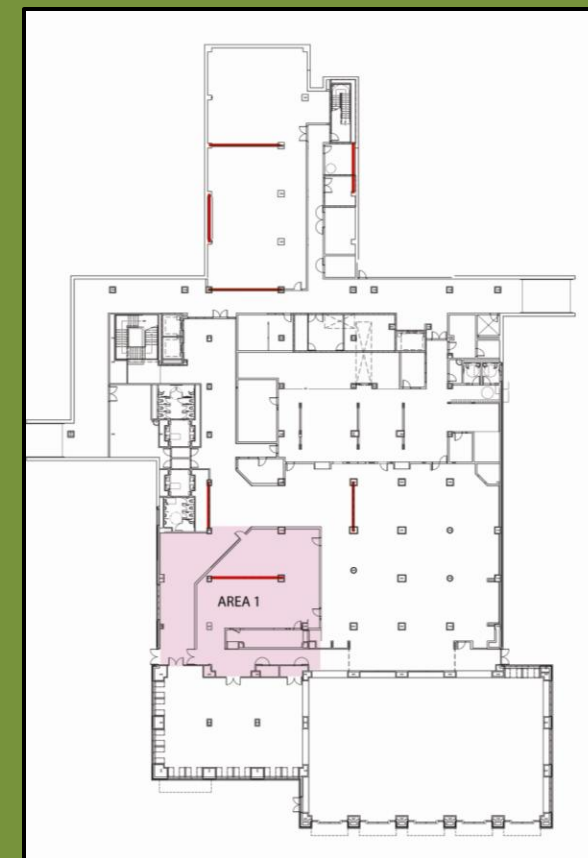


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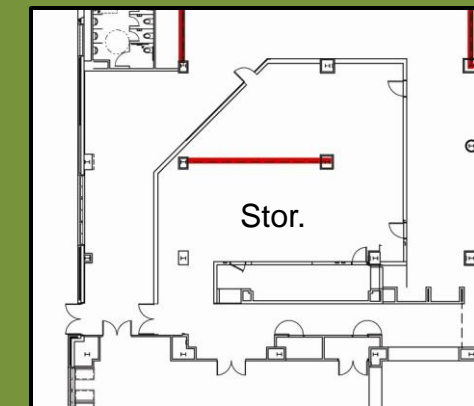
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Image courtesy of SmithGroup



Original Design
750 ft²



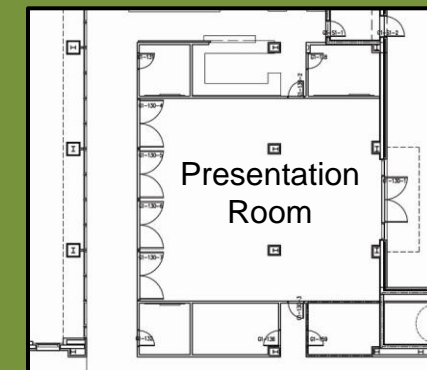
Modified Design
710 ft²

Images courtesy of Jamison Morse

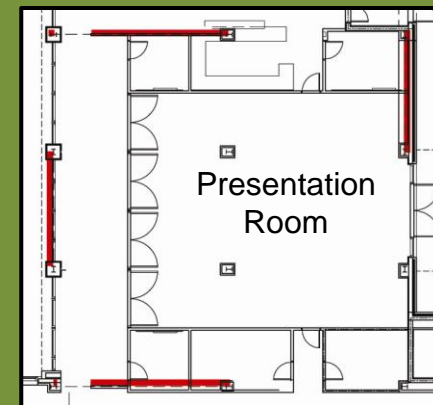
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Original Design



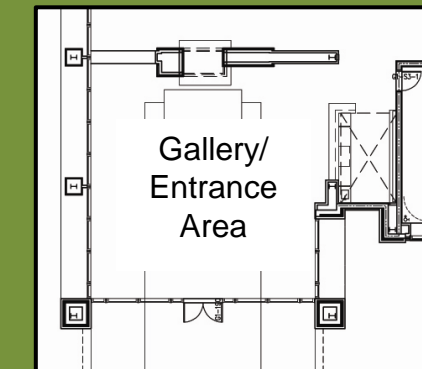
Modified Design

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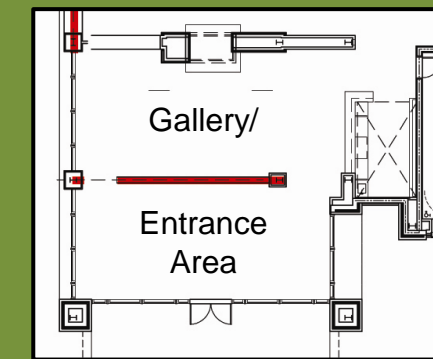
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Original Design



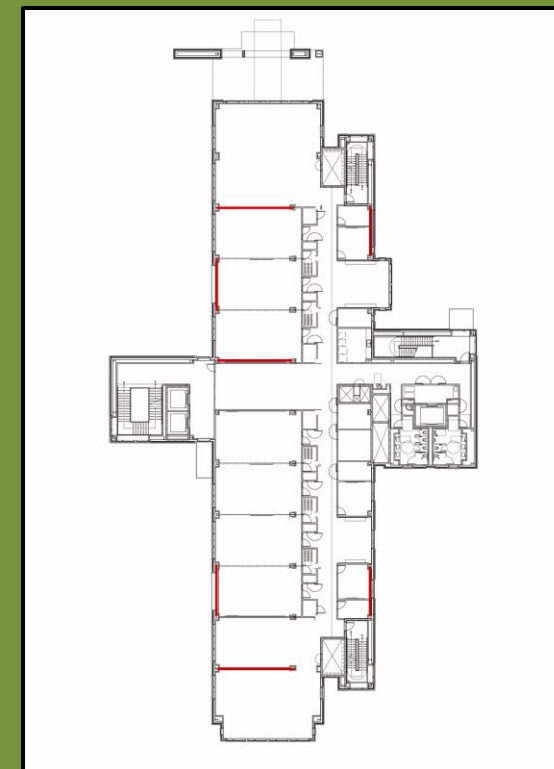
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Image courtesy of SmithGroup



- All braces and columns fit within existing walls and are not obstructive
- Spandrel glass for aesthetics on exterior

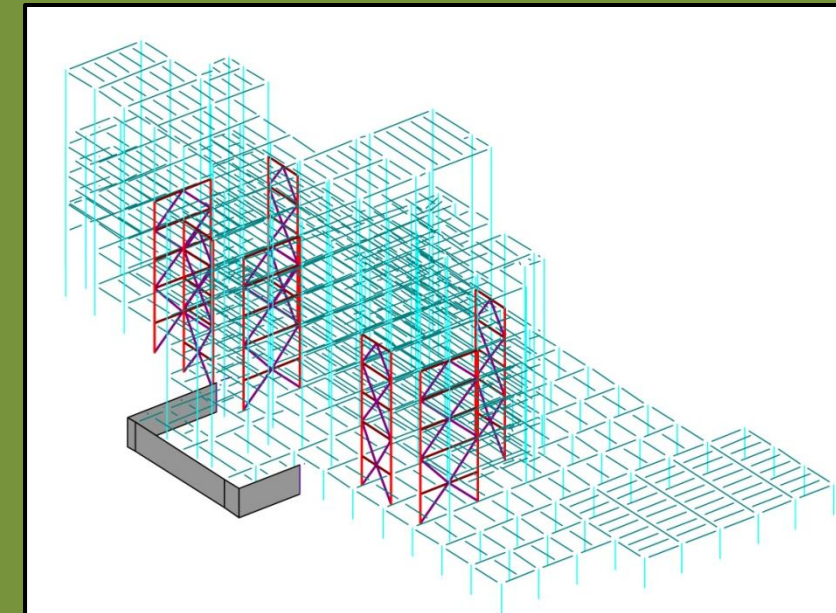
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Image courtesy of SmithGroup

- Cost savings: About \$65,000 in steel
 - Connections savings
 - Smaller column sizes
- Efficient handling of loading
- Feasibly integrated into architecture
- Potential cost savings in the future
 - Minimize damage to columns, beams, non-structural elements
 - Easy to uninstall, test, then reuse or replace



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Image courtesy of SmithGroup

SmithGroup
Star Seismic
Schuff Steel
Thornton Tomasetti
The Penn State University AE faculty
and staff especially:
 Prof M. Kevin Parfitt
 Prof Robert Holland
 Dr. Andres Lepage
My friends and family



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Question and Answer Session